PS Predicting Fracability in Shale Reservoirs*

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Abstract

Effectiveness of fracture stimulation techniques is controlled by the heterogeneous and anisotropic nature of rocks. Sedimentologic and stratigraphic models, the interaction of present-day and induced stress, and rock material properties, which are constrained by composition and mineral distribution, nature of primary sedimentary fabric, and presence and orientation of preexisting planes of weakness, can explain rock behavior.

The Barnett Shale in the Fort Worth basin is dominated by siliceous mudstone to claystone with a clay-mineral and cryptocrystalline quartz matrix. Laminated argillaceous lime mudstone and skeletal, argillaceous lime packstone are also abundant and several other facies are less common. Facies distributions in the central and eastern part of the basin are related to their location relative to advancing shale wedges and differ between proximal, distal, axial, and fringe depositional settings.

Fracability is a function of material brittleness and ductility, which can be inferred from Young's modulus and Poisson's ratio. We calculate values from bulk density and acoustic slowness well-log measurements and incorporate unconfined compressive strength (UCS) and internal friction angle (IFA) strength parameters with these constants. Hand-held penetrometer and micro-rebound hammer (MRH) measurements are used to estimate UCS and IFA. These measurements are performed at a frequency and scale as to be reconcilable with well logs and detailed petrographic, fabric and TOC data and allow calibration of petrographic data and indices of brittleness and ductility with log readings.

To address the behavior of shale reservoirs, we integrate qualitative and quantitative data. These data include core and thin section descriptions, penetrometer and MRH readings, description of present-day stress field, using a stress-strength equilibrium stress polygon approach, which includes estimates of overburden stress, maximum horizontal stress, minimum horizontal stress, and direction of maximum horizontal stress. Using stress estimates, fracture orientation data from borehole image logs, and pore pressure estimates, we calculate pressures needed to reactivate favorably oriented preexisting planes of weakness in shear. This approach establishes a means of calibrating lithology with well log response, allows correlation between lithology and rock strength, and predicts fracability based on these features and the nature of the in situ stress field.

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We view fracability as the ease in which a rock can be forced into supporting a complex fracture network proximal to the wellbore. We define an index of fracability racability index = FI) that is equal to the average of the sum of the Brittleness Index (BI) and Stressed Fracture Index (SFI) so that FI = {Brittleness Index (BI) + Stress

Brittleness (B) is an important rock property. However, no universally accepted concept or measurement method exists (Altindag and Guney, 2010). Models used to uantify brittleness range from the slope of the stress-strain curve after the ultimate strength point has been reached (Jaeger and Cook, 1979) to ratios and ombinations of hardness, fracture toughness, unconfined compressive strength (UCS), tensile strength, Young's modulus, and Poisson's ratio. For expediency, we ave chosen to use the following methodologies and expressions to quantitative indices of brittleness

1. Point Count:

Jarvie's Brittleness = $B_J = (V_{SiO2}) / (V_{clay} + V_{SiO2} + V_{CaCO3})$ Breyer's Brittleness = $B_B = \{(V_{SiO2}) / (V_{clay} + V_{SiO2} + V_{CaCO3} + V_{other})\} X SCM$ SCM = Strength Coefficient Multiplier

 V_{SiO2} = Volume quartz, V_{clay} = Volume clay,

V_{CaCO3} = Volume Carbonate, V_{other} = Volume of other constituents 2. Micro-mechanical testing: From Rickman et al. (2008), referred to here as Mullen (M),

 $B_{\rm M}$ core = {[(E - 1)/7] + [(v - 0.4)/ - 0.25]}/2 E = Young's Modulus (static)

v = Poisson's Ratio

3. Petrophysical log analysis: From Rickman et al. (2008), referred to here as Mullen (M), $B_{\rm M} \log = \{[(E-1)/7] + [(v-0.4)/-0.25]\}/2$

E = Young's Modulus (static)

v = Poisson's Ratio

Fracability = The ease in which a rock can be forced into supporting a > Extract: V_{clay}, V_{SiO2}, V_{CaCO3}, & V_{other}} = B_B & B_J B. Micro Mechanical Data = B C. Log Data = B_M -Stressed Fracture Index (SFI) A. Determine present day stresses via Stress Polygon: S_v, S_H, S_{Haz}, S_h, & P_p}

We also view the interaction of present-day stress with natural fractures and the ease of the fractures to be reactivated during hydrologic fracture stimulation, hereby producing seismic events on a very small scale (e.g. -3 magnitude), as an influential mechanism for the development of a complex fracture network proximal to the wellbore. The ease with which a particular orientation of fractures can be reactivated is quantified here as the stressed fracture index (SFI), which is a function of

Setting / Introduction

• Shale-gas systems show significant differences in lithology and other reservoir properties within basins.

Intrabasinal differences within continuous accumulations account for the indistinctly bound areas of better production termed "sweet spots" by operators.

the fracture reactivation pressure (P_s), pore pressure (P_p), vertical stress (S_v), and net pressure (np). SFI is illustrated in Figure 28.

• Several sets of facies have been recognized in the Barnett Shale in the Fort Worth basin (Breyer et al., 2011).

Work in the central and eastern portion of the basin has related the geographic and stratigraphic distribution of facies within the Barnett to the location of the depositional site on advancing shale wedges (Monroe and Breyer, 2011). Facies in proximal areas of the wedges differ from facies in distal areas, and facies in axial positions differ from facies deposited on the fringes of the wedges.

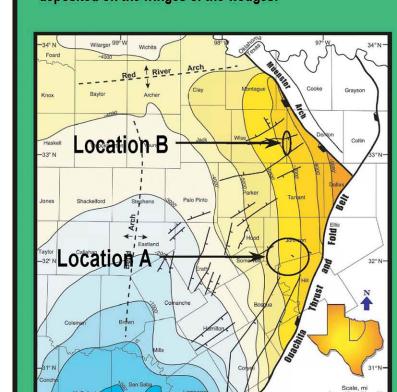


Figure 3. Northwest-southeast cross-section from Johnson County to Hill County (Location A). Green = Barnett Shale; Red colors = middle unit; Blue colors = upper unit. Due to proprietary data, several logs are not displayed in the cross-section (from Monroe, 2009) oughout the stratigraphic tion of the Barnett Shale in the Barnett Shale, middle u

and upper unit in Hill Coun

(from Monroe, 2009)

Barnett Shale: West Johnson County, Texas (Location A)

0 10 20 30 40 50 60 70 80 90 100

Brittleness (B): Core (Mullen) Vs. Log (Mullen)

B, Log, Mullen

UCS, psi,Log

UCS, psi: Core (Dimpler) Vs. Log

re 15. Plot of B_M core vs. B_M log.

gure 16. Plot of UCS core vs. UCS log.

Brittleness Index (BI) . Point Count Data: Brittleness (B) from Core: Jarvie Vs. Mullen Facies Facies A - Laminated spicular mudsto Facies B - Limey layer Facies E - Dark Smooth Mudrock Composition Results of Core Analyses from NW Johnson County

- Dark, siliceous mudstone to claystone with a matrix of clay minerals and cryptocrystalline quartz is the most common facies in the Barnett Shale Calcareous facies are the second-most abundant facies in the Barnett Shale
- With the exception of laminated spicular mudstone (Facies A on Figure 4) all samples generally have high clay

oncentrations (Figure 6).

Facies A - Laminated spicular mudstone
Facies B - Limey layer
Facies E - Dark Smooth Mudrock

Facies E - Dark Smooth Mudrock

unting, 2007).

colored), which are most

undant in the Barnett

Figure 17. UCS and B_M from petrophysical log analyses.

4. Selecting Brittleness Index (BI):

Concentrations of argillaceous materials are dominant (Figure 6). Concentrations of silt-sized particles and siliceous material are greatest near the top of the Barnett Shale (Figure 7).

 TOC concentrations increase toward the lower half of the core (Figure 7). igure 8 is the result of an extraction process that utilizes a form of Neural-Optic processing (Enderlin and Lenzer, 1994)

ich iterates between B_B and B_A with B_M core for the best match using B_M as a reference. A few samples had XRD analysis performed, and the results were used as initial reference points. Figure 9 shows the final results for the iteration between B, and B_{II}, while Figure 10 displays the final match between B_{IM} core and B_{IR}. When it was difficult to achieve a reasonable match Figure 10. Plot of B_{IM} vs. B_{IR}. between B_B and B_I with B_M core, a "scaling Issue" flag was issued (see Figure 1). The same philosophy was used regarding the match between B_M core and B_M log.

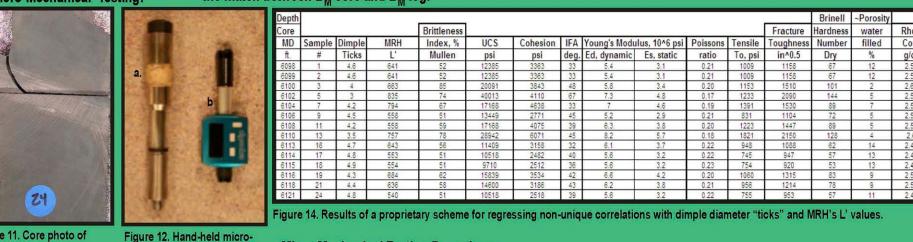


Figure 11 is a photograph of a typical section of core on which micro-mechanical testing was performed. a. Dimpler, b. micro rebound Dimpler: Compliant tape is placed on the core sample. The conical tip of the Dimpler (Figure 12a)

indenter is coated with a (red) dve and then forced, at a constant axial load, through the tape and into the core sample creating a conical red marked depression (dimple) on the tape (Figure 13). (See Ramos et al. (2008) for operational concept of the Dimpler). The tape with dimple marks is removed and placed on clear plastic so that the front side and adhesive side of the tape are visible for analysis (Figure 13). A surface magnifier, scaled in ticks, is used to measure the Dimple diameters. Diameters are converted into an index of Unconfined Compressive Strength (UCS) (See Enderlin and Alsleben (in press) or Enderlin et al. Micro Rebound Hammer (MRH): The MRH (Figure 12b) is placed upon the core sample and the impactor

shuttle is activated. The velocity with which the impactor shuttle rebounds off the core surface is measured as an L' number. See Zahm and Enderlin (2010) for relationships of L' to UCS.

Analysis of Micro-Mechanical Testing:

egressing with published correlations from Deere and Miller (1996), Weingarten and Perkins (1995), Rickman et al. (2008) and Gidley et al. (1989) with Dimpler derived UCS and MRH L' values yield non-unique solutions for the attributes found in Figure 14. Figures 15 and 16 show the relationship between core B_M and UCS with B_M and UCS from logs.

In this case, the determination of rock strength from log measurements requires fluid substitution to account for the presence of overpressured gas and then project the log values to 100% water saturation (S_W = 100) at a normally pressured state. Once accomplished, strength evaluation was performed. Figure 17 displays a few feet of log input data and outputs of unconfined compressive strength (UCS) and B_M log. Figure 18 presents UCS from log, UCS

Figure 18. Comparison of UCS and Rho, derived from micro-mechanical testing and log measurements.

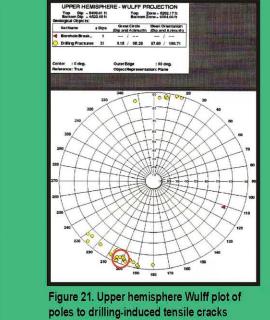
B, Jarvie,Cor

from core along with their respective bulk density

Figure 19. Comparison of brittleness (B_J, B_B, B_M) derived from point counting, micro-mechanical testing and log measurements. For the interval displayed, a BI of ~55% would be a reasonable value.

It is worth noting the spread of BI values at similar depth as a function of the measurement method's support volume. Micromechanical testing, thin section point counting, and logging provide insight into the complexity of the lithology and potential scaling issues (see Figure 1). For example, the interval between 6100 and 6125 ft contains considerable scatter whereas the section around 6200 ft is relative behaved. The upper section contains numerous strong phosphate nodules (see 6102 ft in Figure 19).

Characterizing Present-Day Stress:

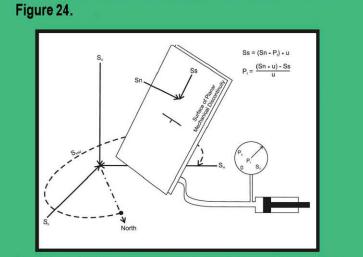


direction (SHaz) is ~115°. Using the methodology outlined in Zoback (2007) with the strength data from the previous section, borehole image information (Figure 20 and 21), particularly the presence of drilling induced tensile cracks, pore pressure data, value for the coefficient of sliding friction (u) and mud weight information, the present-day stress state is graphically estimated via a

Project the subsurface stresses (S_V, S_H, S_h) onto the surfaces of mechanical discontinuities, spatially defined by strike and dip, and resolve the normal and shear stresses. See Figure 23. Calculate, as a function of strike and dip, the fluid pressure required to make the product of the normal stress, less fluid pressure, and u equal to the shear stress as described by Barton et al. (1995). Values are presented in

Determining Fracture Reactivation (Reactivation Table - RT):

stress polygon (Figure 22).



described by strike and dip is placed in a stress field described by Sys S_H, S_h, and S_{Haz}. The normal stress (Sn) and shear stress (Ss) can be calculated. Reactivation occurs by increasing P, such that the product of (Sn - P_f) and u is less than Ss.

. Observed Fractures:

Figure 24 displays the P, for selected strike-dip pairs. The task is to find which pairs exist in the fracture of The fracture data may be derived from field observations, core inspection, microscopic investigation, seismic studies, and petrophysical well logs. In all cases both strike and dip must be resolved. Here, the results of "poles to planes" are derived from the analysis of a borehole electric imager log and are plotted in Figures 2 and 26. The red arrows show the association between reactivation table and poles to planes.

Determining Stress Fracture Index (SFI):

The calculation of SFI differentiates

observed fractures that are inconsequentia

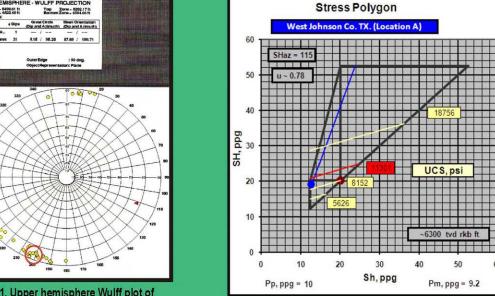
favorable and detrimental to the development of a complex fracture networ proximal to the wellbore via hydraulic igure 27. Plot of reactivation pressure (Pf) as a function of strike and dip [Reactivation Table (RT fracturing fluids. Inconsequential fractures are those with P, higher than the sum (A) of S, and net-pressure, which, therefore, cannot be reactivated. Fractures, like faults, with a P_r close to P_n would easily reactivate, thereby diverting the fracturing fluid, and never allow the fracturing pressure to exceed the S_b to form Mode I fractures. Pressure B is the

detrimental threshold pressure. Favorable fractures are those with P, between A and B and normally reside near the interface on the yellow and blue cells on Figure 27.

. Determining Fracability Index (FI):

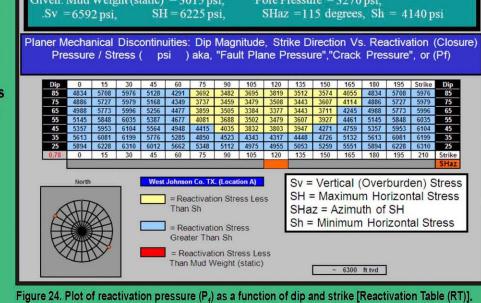
Using the value of SFI from Figure 28 and BI from Figure 19, the NP = 800 psi, the SFI = 20%. FI is calculated. FI = (BI + SFI) / 2 = (55% + 20%) / 2 = 38%

Stress Fracture Index (SFI)



lines and their UCS values plotted. The yellow rock strengt lines are positioned for reference only. The red line and trength of interest (11300 psi). S, is identified by the dark re

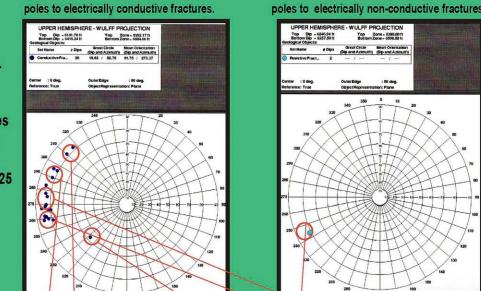
Figure 22. Stress Polygon with four rock strength contour



than Sh; Blue: Reactivation at pressures that are greater than Sh; Red: Reactivation pressure less

Figure 26. Upper hemisphere Wulff plot of

→ Good Inconsequential



Model Equations for Stressed Fracture Index (SFI)

 $FI = \{1 - [1/Sum:(Positive Influence)]\} - \{1 - [1/Sum:(Negative Influence)]\}$

Pf = Fracture reactivation pressure, Pp = Pore pressure, Sh = Minimum horizontal stress, Sv = Vertical stress, NP = Net-pressure, A = (Sh + NP), B = Fault avoidance factor

Figure 28. Expression for the calculation of the SFI. With B = 400 psi, and

 $1/\{(B - Pf)/(B - Pp)\}$ $1/\{(A - Pf)/(B - A)\}$

Negative Influence Positive Influence

Figure 25. Upper hemisphere Wulff plot of

treating pressure to a value above Sh.

roduction reference (PR) = A3MP / (product of MT, TVP, and NS)

MCF / (MMgal ft NoS)....

Brittleness Index (BI)

Micro-mechanical data (Figure 29) was

collected and analyzed with the results

appearing in Figure 30. Petrophysical log analysis was conducted with the data

available. Both the data and results are

Figure 31. UCS and B_M from petrophysical log analysis of data provided by

Selecting Brittleness Index (BI): Data from

core and logs suggest a BI value of ~ 46%

Fracability Index, as defined here, provides a numerical method for evaluating fracturing effectiveness by combining stratigraphic observations (particularly facies

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Stress Fracture Index (SFI) 1. Characterizing Present-Day Stress:

Stress Polygon (Figure 32) was developed with the data from Lancaster (1992) to evaluate the presentday stresses. Reactivatio table (Figure 33) was constructed from the stress information. 10 20 30 40 ppg = 10 S_h, ppg P_m, ppg = 9.6

2. Determining Fracture Reactivation (Reactivation Table - RT):

Barnett Shale: T.P. Sims #2, Denton-Wise County Line, Texas (Location B)

3. Observed Fractures:

Observed fractures (Figure 34) are projected on to the reactivation table (Figure 33) and the SFI and FI are calculated.

5. Determining Fracability Index (FI):

4. Determining Stress Fracture Index (SFI): With B = 400 psi, and NP = 800 psi, the SFI = 90%

FI = (BI + SFI) / 2 = (46% + 90%) / 2 = 68%

Production Reference (PR)

Production was evaluated by using data from the Texas Railroad Commission database particularly from the "Gas Well Back Pressure Test, Completion or Recompletion Report, and Log" Form G Average of the highest first 3 months production (A3MP), MCF... Measured thickness of interval(MT), ft ...

Fracability Index (FI) vs. Production Reference (PR)

On the left is the comparison of Production Reference and Fracability Index, FI = (BI + SFI)/2, for the two locations. Whereas on the right is a comparison

. Once a fracture's P, has been exceeded by pressure of the hydrologic fracturing fluids, shear failure occurs (activation). When pumping is stopped and the pressure of the hydrologic fracturing fluid falls below the P. (deactivation), the "fall-off" pressure response, as a function of time, can be misconstrued as "closure" pressure (see Couzens-Schultz and Chan, 2010, also Mullen and Enderlin, 2010). Implying that traditional "closure" pressure value may or may not be equivalent to Sh. Fractures with low P, (center of the yellow sections of Figures 24, 27, and 33) need to be forced into a "tip-screen-out" condition in order to increase the bottom hole

variations), rock properties (with emphasis on brittleness), natural fracture orientations (dip and strike), present-day stresses, pore pressure, and bottom hole hydrologic

. The method presented also provides insights into the scaling issues regarding rock properties and stress distribution.

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Number of stages (NoS)

Implication